

NTNU – Trondheim
Norwegian University of
Science and Technology

PhD Outlines

Benoit Loranger

Frost susceptibility of crushed rock aggregates and
field assessment of frost heave and frost depth

Supervisor: Pr. Inge Hoff

Co-supervisors: Pr. Guy Doré and PhD Elena Scibilia

Background

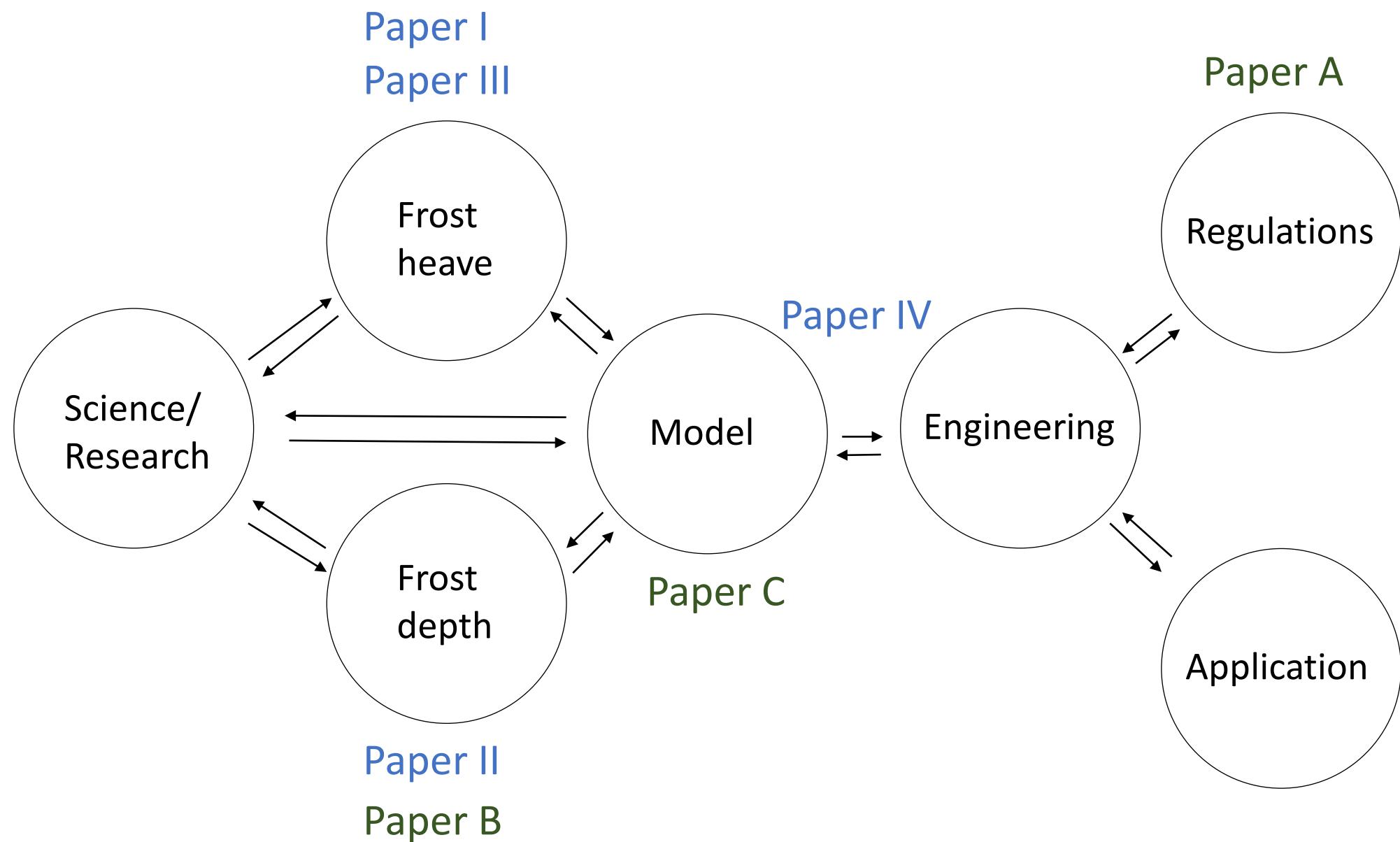
- Winters 2009/10 and 2010/11
- Revision N200 in 2014
- Question related crushed rock aggregates behaviour

Background

- FROST project
 - Initiated in 2015
 - 2 Ph.D.'s:
 - Karlis Rieksts (thermal considerations of CRA)
 - Myself (Frost susceptibility)
 - 4 Ph.D.'s on crushed rock aggregates
 - Marit Fladvad (Optimization use in road construction)
 - Diego Maria Barbieri (Use of local materials)

- What is the frost susceptibility of crushed rocks?
- How the frost heave affect main and secondary roads?
- What is the effect of having different frost protection layer composition?
- Is a model adapted to predict frost depth and frost heave in thick road structures





Part I: Laboratory investigation

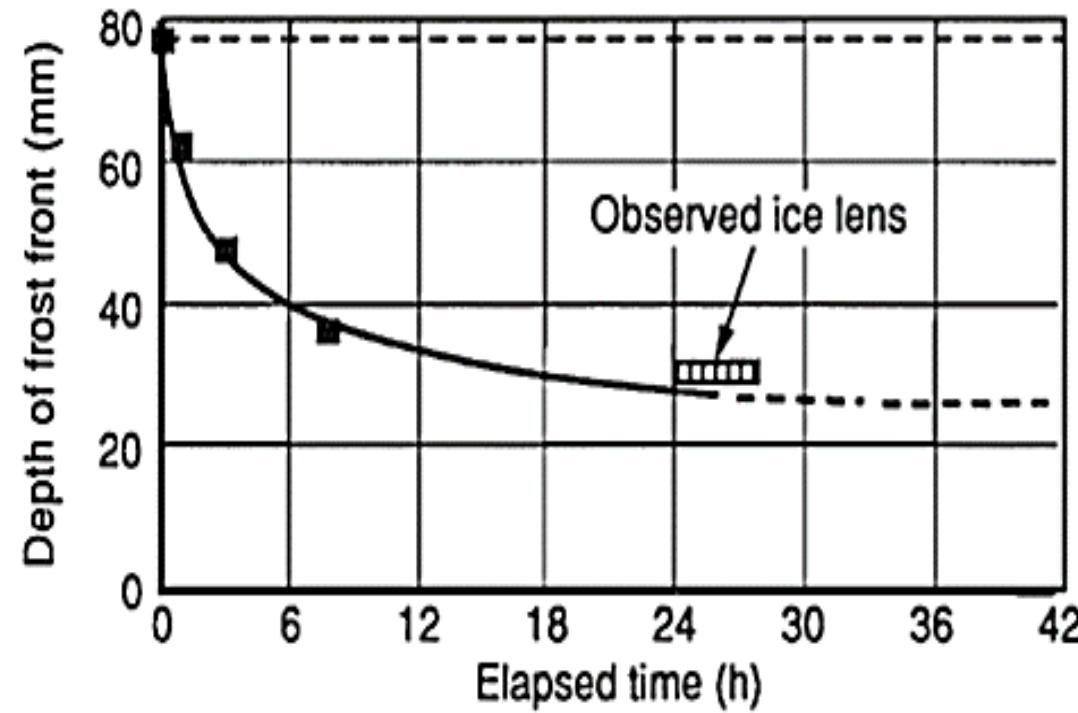
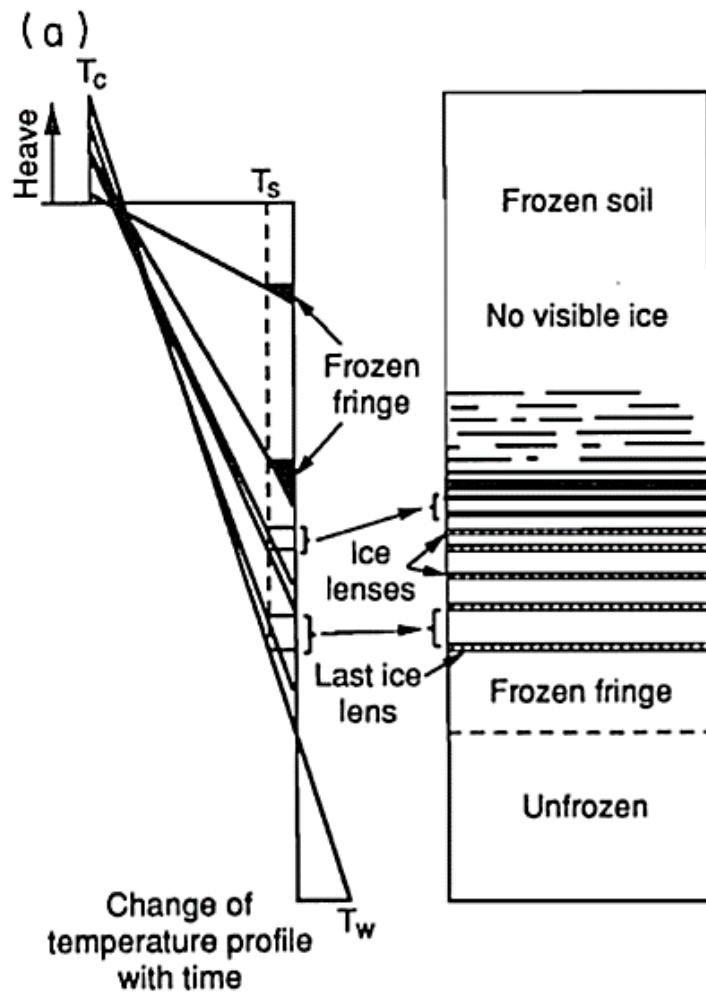
- Paper I: Frost heave laboratory investigation on crushed rock aggregates.

Québec 2019: 18th International Conference on Cold Regions Engineering and the 8th Canadian Permafrost Conference. Quebec City, Canada. (August 18th to 22nd 2019).

- Paper III: Assessing soil index parameters to determine the frost susceptibility of crushed rock aggregates.

Re-submitted with minor correction to Cold Regions of Science and Technology Journal (November 10, 2021).

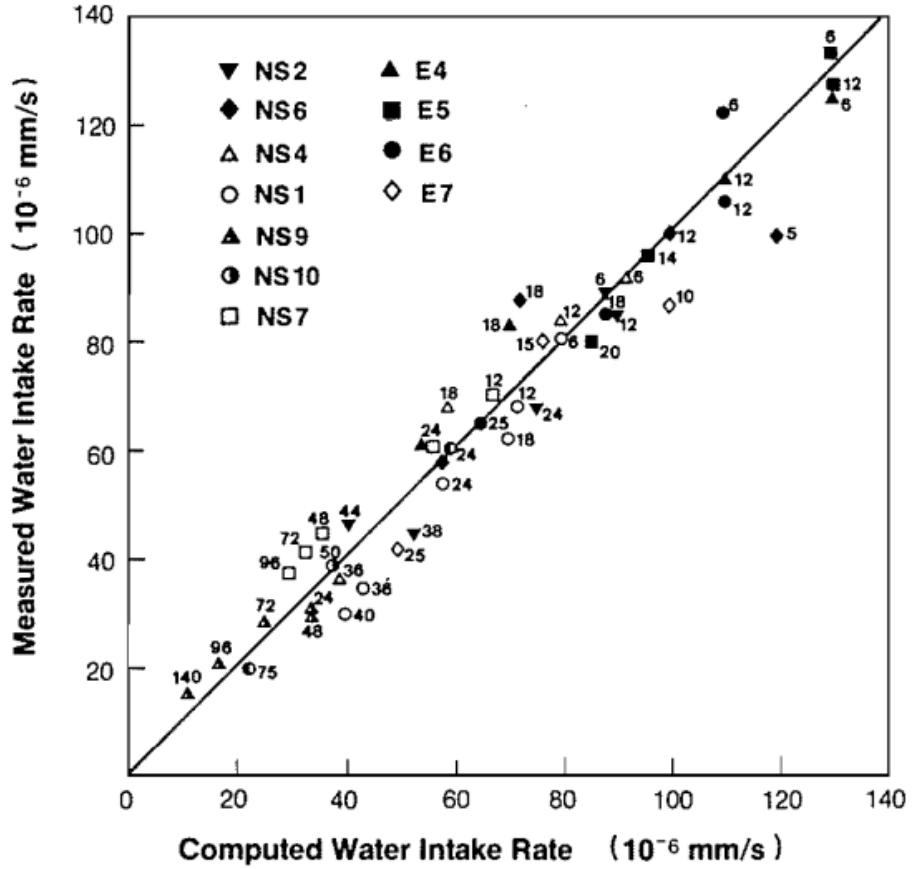
Frost heave



Test procedure



Segregation potential



$$v_u = \frac{P_w - P_u}{d} \bar{K}_f = \left(\frac{P_w - P_u}{T_s} \bar{K}_f \right) \text{Grad } T = \text{SP Grad } T$$

$$dh = dh_p + dh_s$$

Where : $dh_{sp} = 1,09 \cdot Sr \cdot n \cdot dt$

$$dh_s = 1,09 \cdot SP \cdot \nabla T_f \cdot dt$$

Granite/ gneiss are within the range found in literature (Konrad 2005; Nurmikolu 2005)

Adding reference values for granodiorite, porphyr, anortosite, slate and gabbro

Quarry	rock type	< 63 µm %	< 20 µm %	< 2 µm %	SP mm ² /°C·d
Aplitt	Granodiorite	12.5	7.4	1.23	149
Hadeland	Porphyry	16.1	9.7	2.30	176
Hellvik	Anortosite	22.4	13.2	2.45	197
Legruvbakken	Slate	25.6	11.5	0.56	60
Vassfjell	Gabbro	24.9	14.8	2.05	111
Lørenskog	Gneiss	16.2	7.1	0.53	105
C.S. Lør.+ Velde	Gneiss/Granite	20.0	9.1	1.41	130
Velde	Gneiss/Granite	30.7	15.2	2.83	197

Frost susceptibility	Class	Material < 22,4 mm		
		Mass %		
		< 2 µm	< 20 µm	< 200 µm
Non/ Negligible	T1	--	< 3	--
Low	T2	--	3-12	--
Moderate	T3	>40 ¹⁾	> 12	< 50
High	T4	< 40	> 12	> 50

1) Soils with more than 40% of < 2 µm are considered as moderately frost susceptible (T3).

frost-susceptibility	SP mm ² /°C·h	SP mm ² /°C·d	Frost heave rate mm/d	Frost heave ratio Δh/h _{frozen} (%)
Negligible	< 0.5	< 12	< 0.5	< 1
Low	0.5 - 1.5	12 - 35	0.5 - 2	1 - 4
Medium	1.5 - 3	35 - 75	2 - 4	4 - 8
High	3 - 8	75 - 200	4 - 8	8 - 20
Very high	> 8	> 200	> 8	> 20



Soil index parameters (Konrad 2005)

Soil index parameters to measure

Average diameter of the fine fraction ($d_{50,ff}$)

-> hydrometer

Initial water content -> in situ

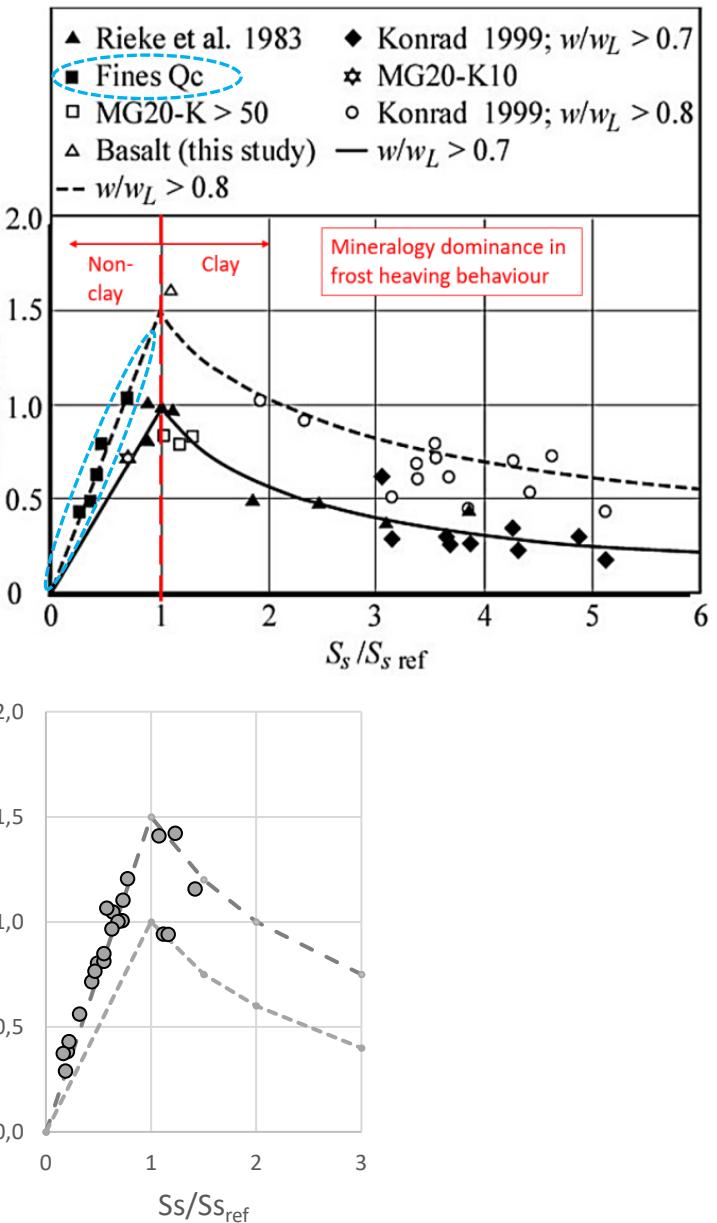
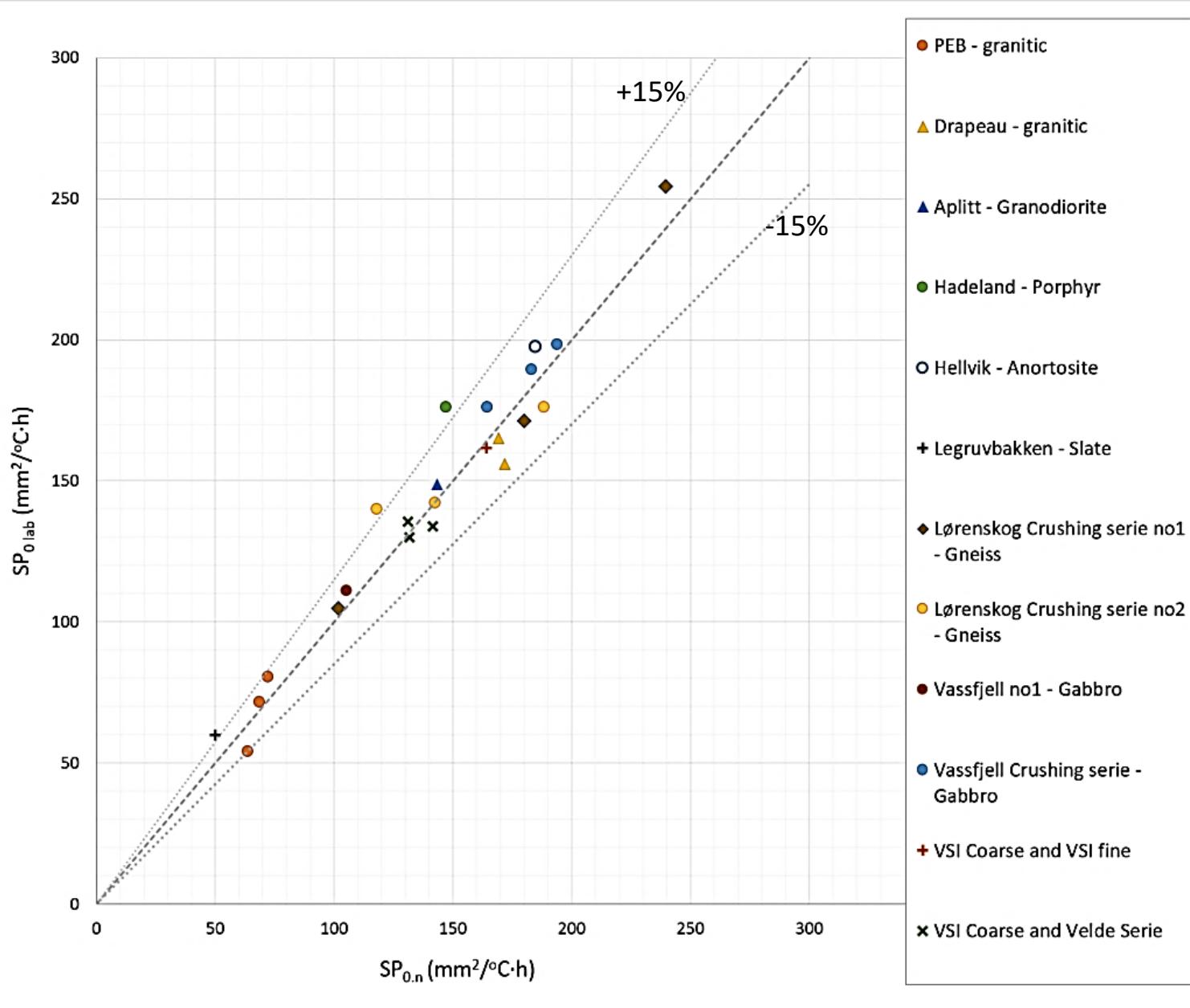
Liquid limit -> cone penetration

Specific surface area -> methylene blue test

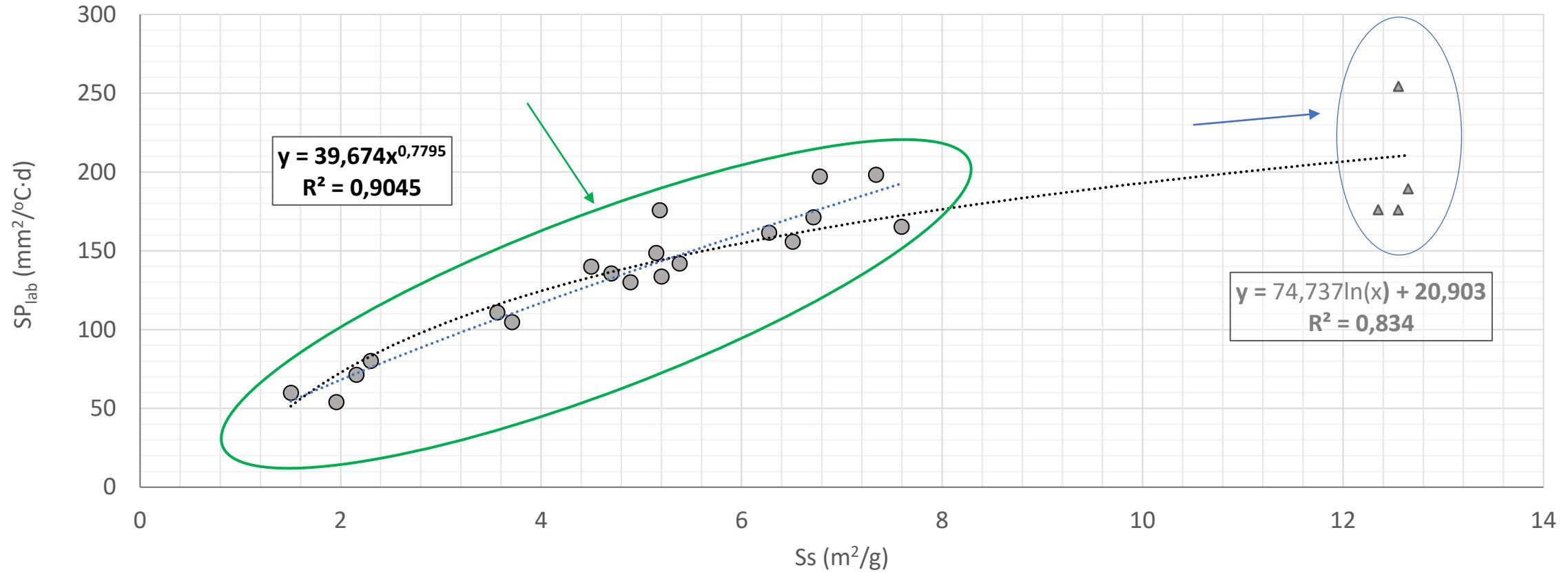
23 frost heave tests

variation in rock type

variation in crushing step



Last stage crushed aggregates



Part II: Field test investigation and modelling

- Paper II: Frost depth and frost protection capacity of crushed rock aggregates based on particle size distribution.
Québec 2019: 18th International Conference on Cold Regions Engineering and the 8th Canadian Permafrost Conference. Quebec City, Canada. (August 18 to 22, 2019).

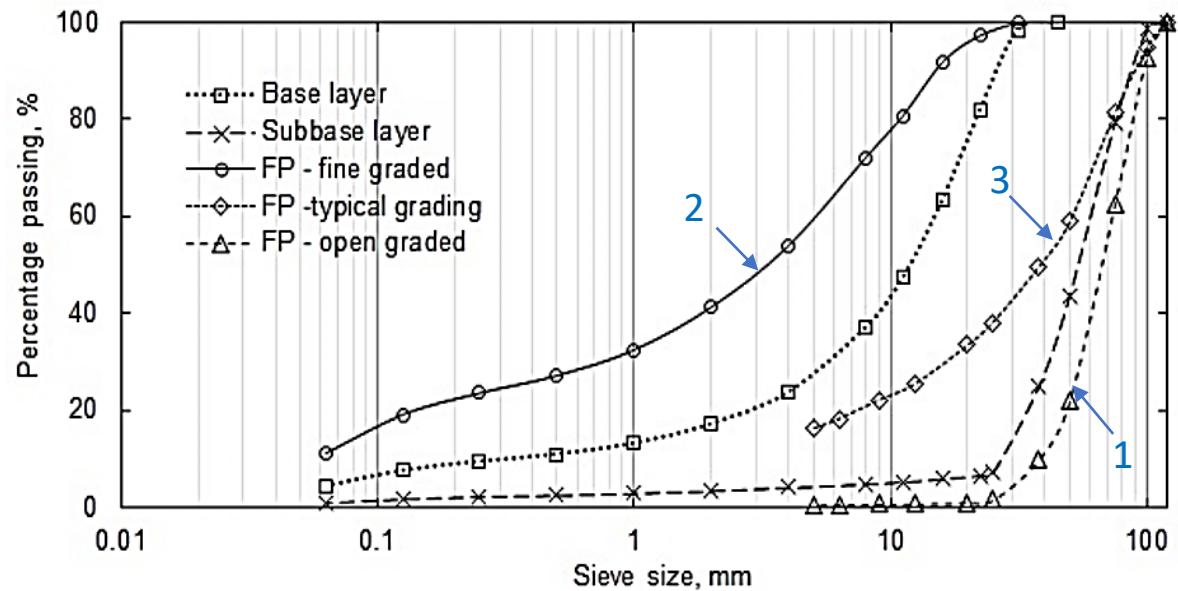
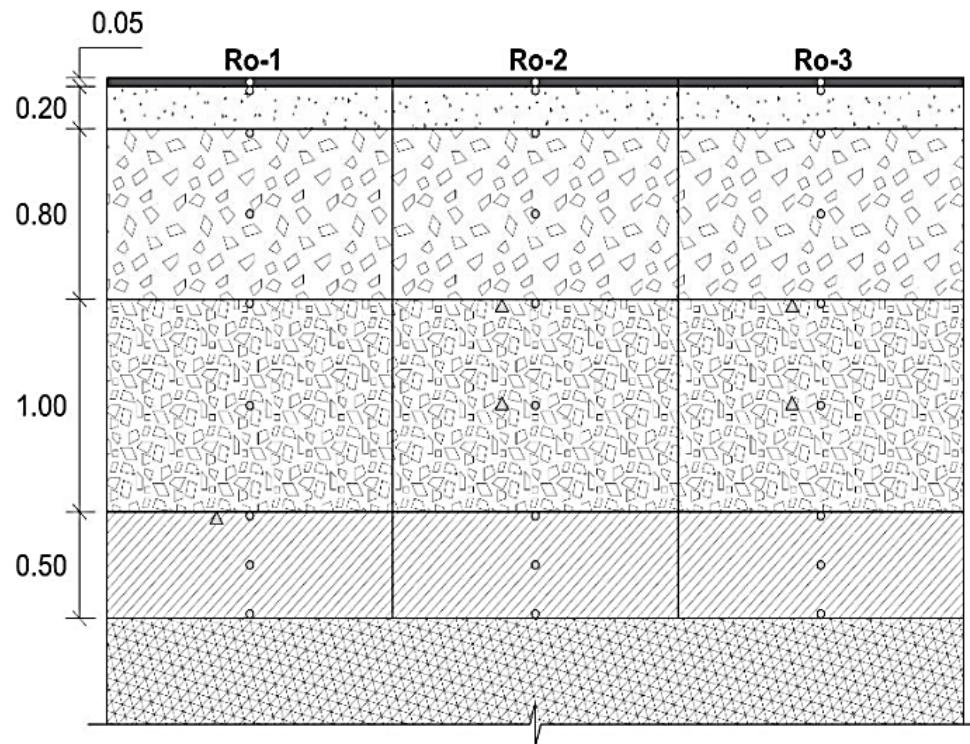
- Paper IV: Evaluation of the SSR model on thick-layered road structures using the I3C-ME frost module and analyses of key parameters.
To be re-submitted.

Field test site: Røros



Locality	MAT	FI ₂	FI ₁₀	FI ₁₀₀
Røros	0.2	21000	39000	61000
Oslo	6.4	5000	12000	21000
Bergen	7.6	1000	2000	4000
Trondhein	5.3	4000	11000	19000
Tromsø	2.8	8000	15000	24000

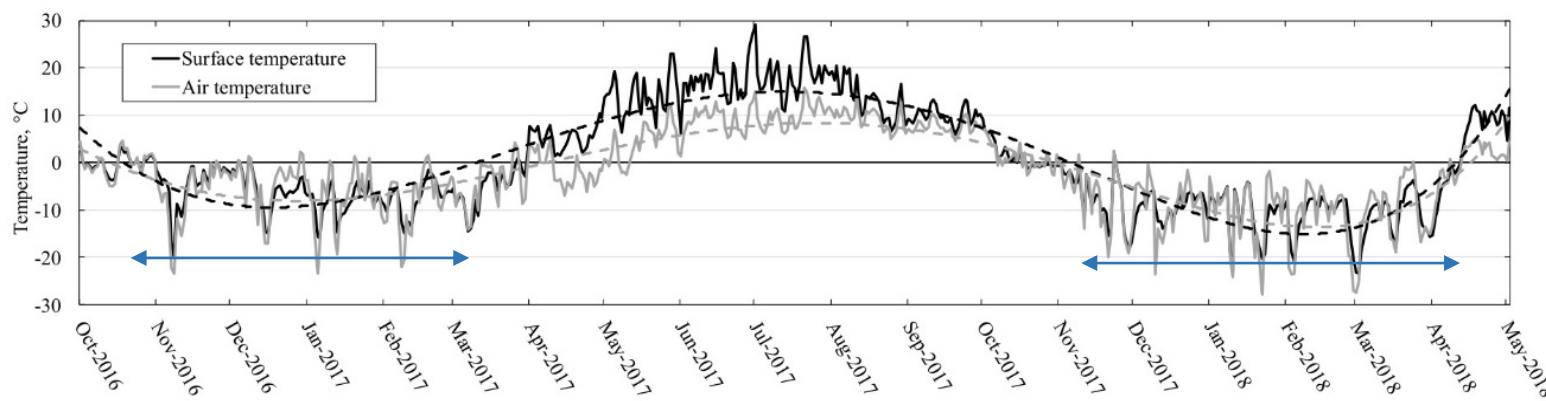
FI_x: Return period in x years of a freezing index magnitude event



Legend:

- asphalt 0/8
- base 0/32
- subbase 0/120
- frost protection*
- artificial subground (silt)
- natural subground (clay)
- - temperature sensor
- △ - moisture sensor

* see Fig. 2 for particle distribution

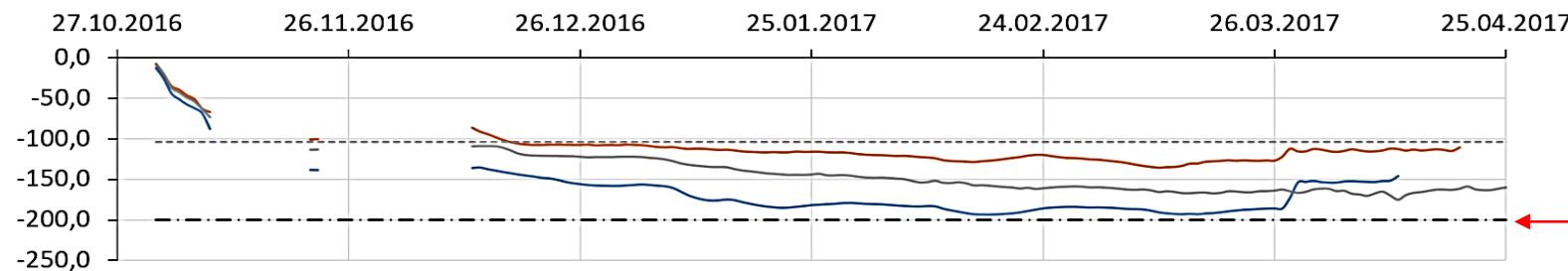


$$FI_s = 22630 \text{ } ^\circ\text{C}\cdot\text{h} \text{ // } 943 \text{ } ^\circ\text{C}\cdot\text{d} \approx F_3$$

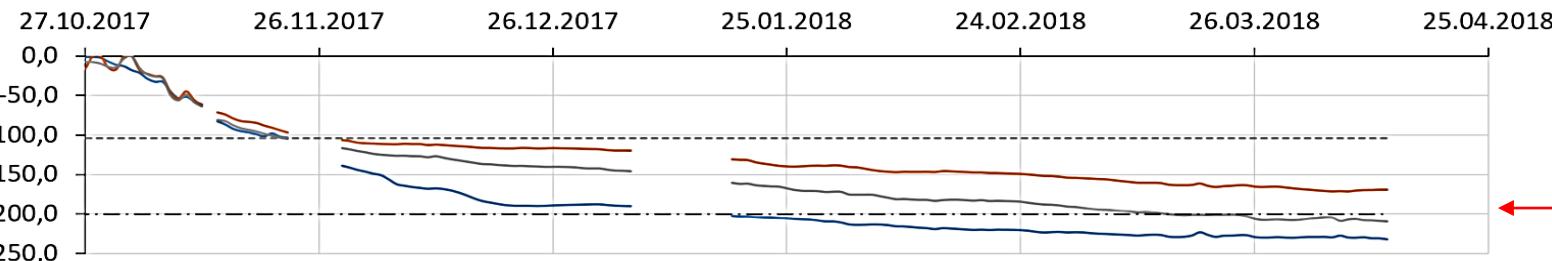
$$FI_s = 36683 \text{ } ^\circ\text{C}\cdot\text{h} \text{ // } 1528 \text{ } ^\circ\text{C}\cdot\text{d} \approx F_9$$

— Ro1 — Ro2 — Ro3 ----- Contact FPL --- Contact silt

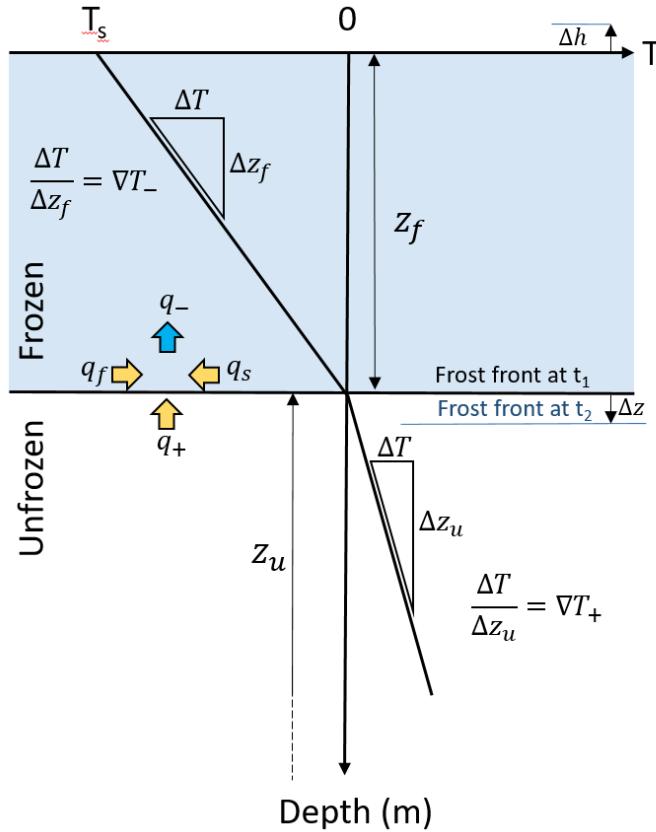
Winter 2016-17



Winter 2017-18



SSR Model



Energy balance at frost front

$$q_- = q_f + q_+ + q_s$$

$$\lambda_f \frac{\Delta T}{\Delta z_f} = L_s \frac{\Delta z}{\Delta t} + \lambda_u \frac{\Delta T}{\Delta z_u} + SP \cdot L_w \frac{\Delta T}{\Delta z_f}$$

$$\lambda_f \nabla T_- = L_s \frac{\Delta z}{\Delta t} + \lambda_u \nabla T_+ + SP \cdot L_w \nabla T_-$$

Stefan
(1891)

Saarelainen
(1992)

Konrad
(1980)

$$\Delta h = \Delta h_s + \Delta h_p$$

$$\Delta h_s = 1.09 \cdot [SP \cdot \nabla T_-] dt = 1.09 \cdot SP \cdot \frac{(T_f - T_s)dt}{(\lambda_{fz} \sum(z_i/\lambda_{fi}))}$$

$$\Delta z = (T_f - T_s)dt \frac{(1 - SP \cdot L_w / \lambda_{fz})}{(L_{fz} \cdot \sum(z_i / \lambda_{fi}))} - \frac{S \cdot \nabla T_+ \cdot \lambda_{uz} dt}{L_{fz}}$$

Pavement structure		Geotechnical input parameters				Thermal input parameters						Calculated input parameters				
Material	Thickness (mm)	ρ_d (Kg/m³)	ρ_s (Kg/m³)	Water content (%)	S_s Sol (m²/g)	K_s (W/m.K)	x (W/m.K)	η	$Kappa_u$	$Kappa_f$	a (1/MPa)	S_{Po} (mm²/KH)	S_r (%)	L_f (Wh/m³)	K_u (W/mK)	K_f (W/mK)
1 Asphalt mixture	80	2400	2650	0,0	0	2,5	0	0	0	0	15	0	0,0	1250	1,66	1,66
2 MG 20	190	1980	2788	4,0	0,5	3,3	1,7	1,8	4,7	1,8	15,0	0,0	27,3	7348	1,47	1,49
3 MG 56	840	1625	2788	1,0	0,5	3,3	1,7	1,8	4,7	1,8	15,0	0,0	3,9	1508	0,51	0,47
4 MG 56	600	2116	2788	5,5	0,5	3,3	1,7	1,8	4,7	1,8	15,0	4,1	48,3	10798	1,90	2,11
5 MG 56	300	2116	2788	5,5	0,5	3,3	1,7	1,8	4,7	1,8	15,0	4,1	48,3	10798	1,90	2,11
6 MH (IL< 0,9)	500	1600	2750	26,1	10,0	2,7	0,8	1,2	1,9	0,9	7,0	8,0	99,9	38744	1,44	2,39
7 CL	0	1300	2750	34,0	25,0	2,7	0,8	1,2	1,9	0,9	7,0	8,0	83,8	41008	1,13	1,90

Basic climatic data

Average annual air temperature (°C) :

1.10

Air transfer coefficient ->Surface :

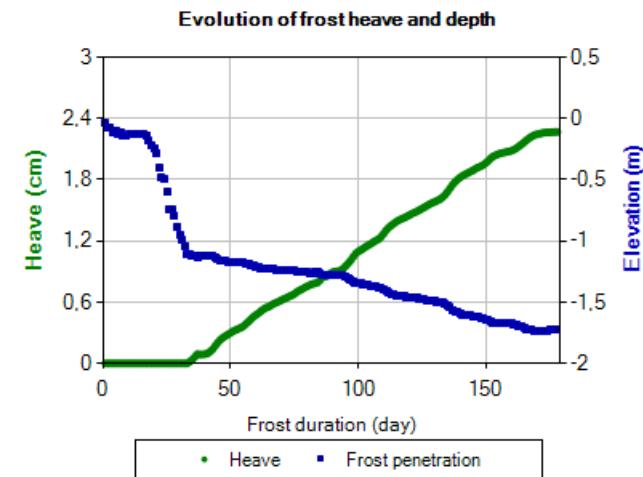
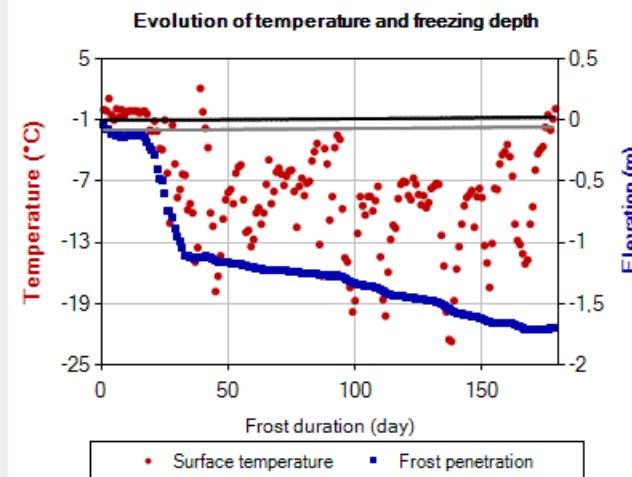
0.99

Method 1: Sinus Simulation Method 2: Monthly calculation Method 3: Daily entry

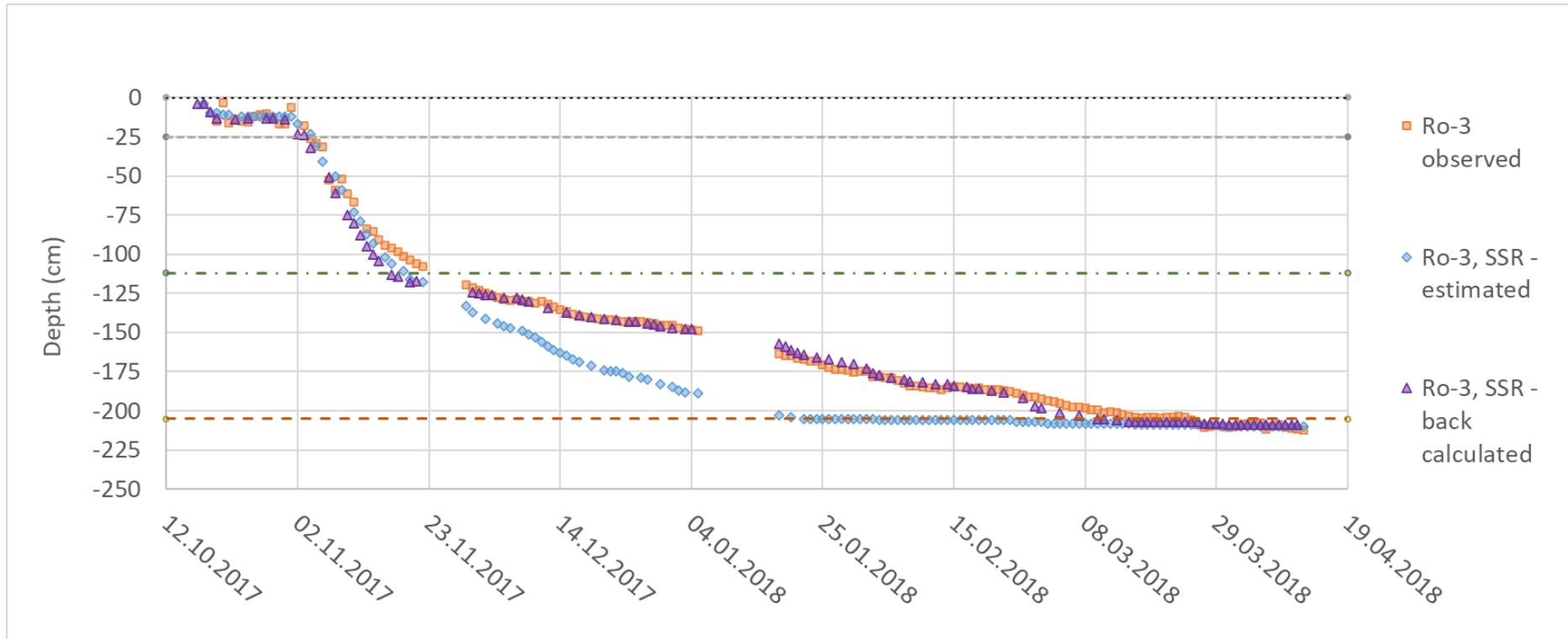
Method 3: For each day, indicate the duration and temperature of the air. Calculation will be done for each time and temperature increment.
Or select a weather file on your computer:

Open Weather File

Period N°	Duration (Hour)	T° (air)	Ts
1	24	0	,0
2	24	-,10	-,1
3	24	1,10	1,1
4	24	-,50	-,5
5	24	-1,00	-1,0
6	24	,10	,1
7	24	-,70	-,7
8	24	,00	,0

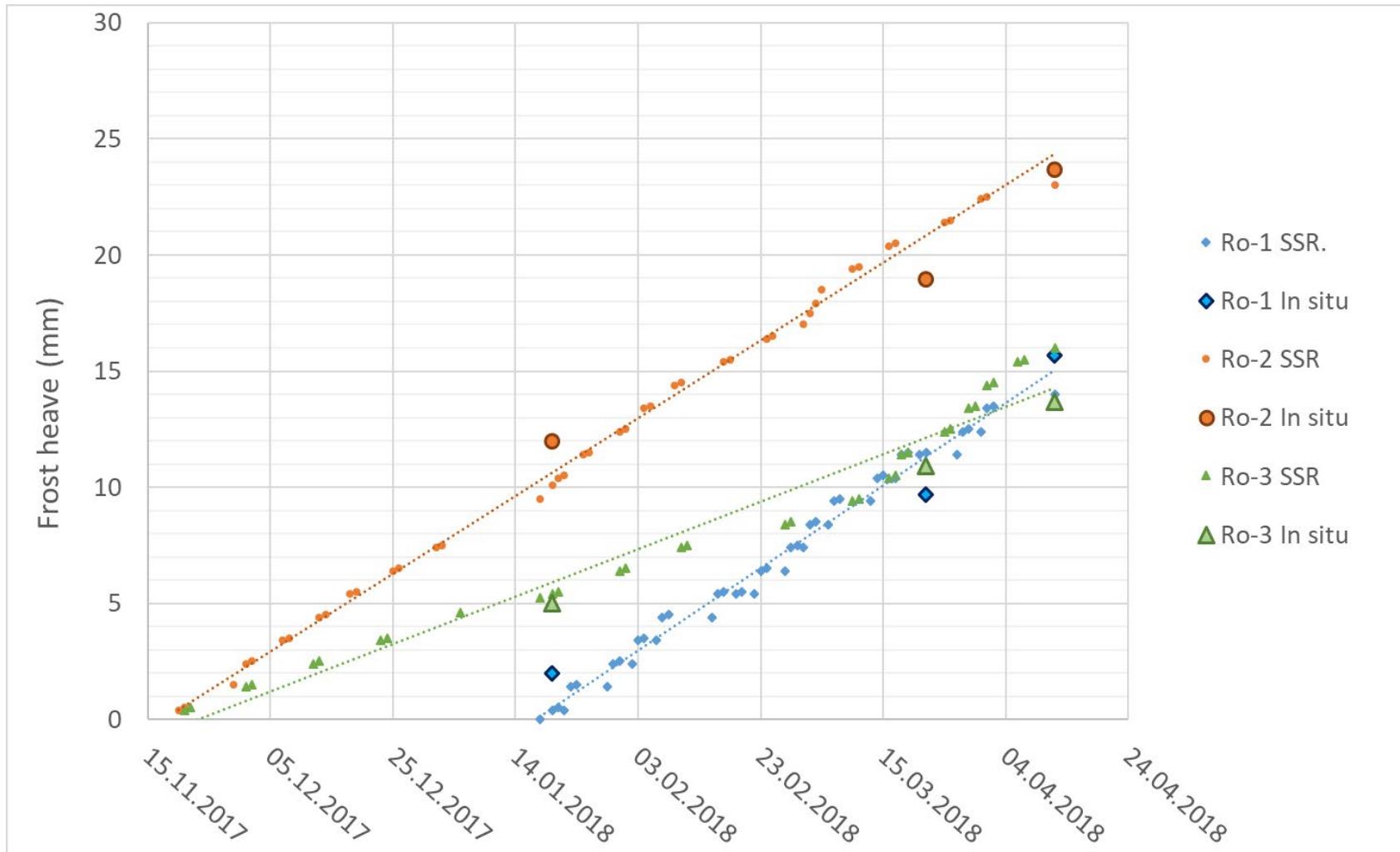
VALIDATE
THE MODULE

Frost depth: Ro-3



Section (FPL)	Water content (%)		Dry density (kg/m^3)	
	est.	adj.	est.	adj.
Ro-1	1	1.5/2.5	1535	1535
Ro-2	5.5	5.5	2116	2116
Ro-3	2	4/4.5	1780	1850

Frost heave



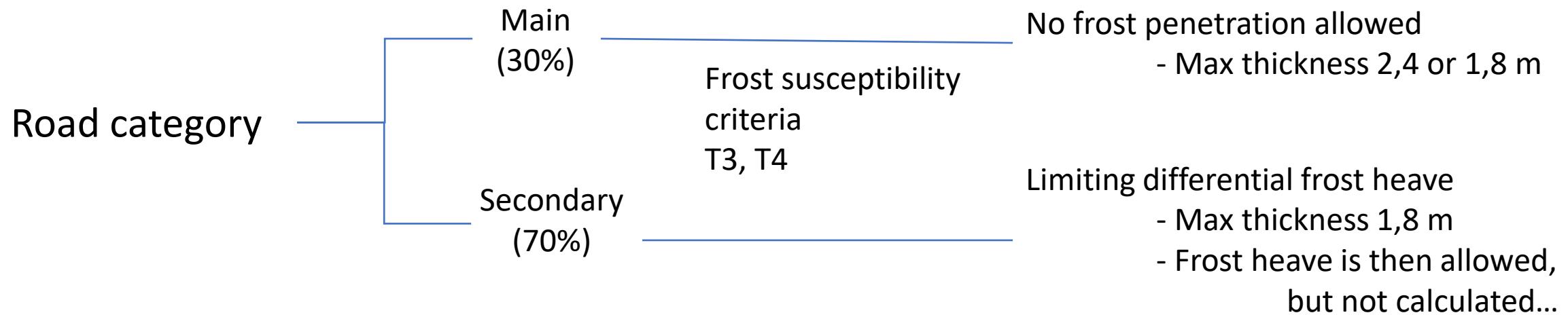
Silt tested in laboratory

Granular material
(Ro-2 and Ro-3) are back-calculated ($\text{mm}^2/\text{K}\cdot\text{h}$)

$$SP_0 \text{ Silt} = 8.1$$

$$SP_0 \text{ gm-Ro-2} = 4.1$$

$$SP_0 \text{ gm-Ro-3} = 1.3$$



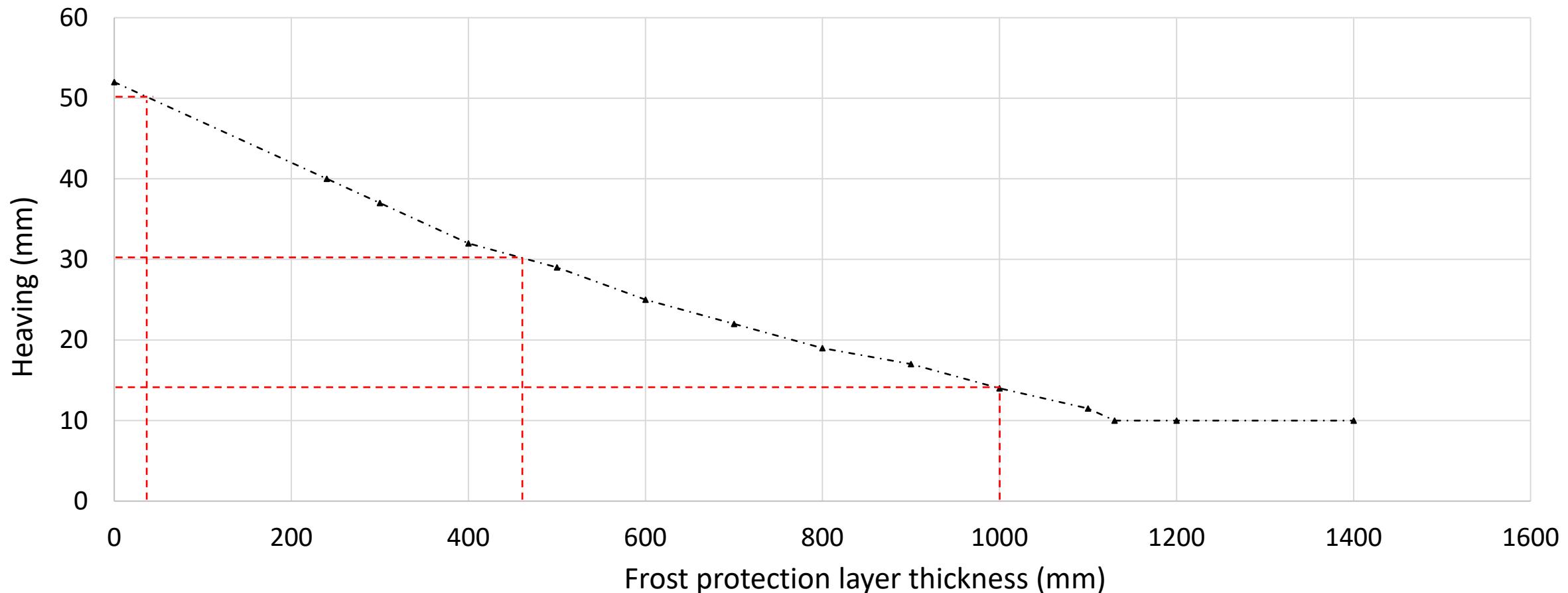
Road class	Freeways	National	Regional	Local
Finland	<30	<50	---	<100
Canada	<50	<55	<60	<70

Speed limit	110	100	90	70	<50
Sweden	<20	<50	<80	<120	<160

Doré and Zubeck, 2009

Design optimization, Ro-3

Varying the FPL thickness
Everything else's the same; Fls $\approx 35000^{\circ}\text{C}\cdot\text{h}$



Continuations

- Post doctoral project with Statens Vegvesen
- VegDim and EraPave,
 - EraPave
 - Soil and aggregates parameters list
 - SSR ASSESSMENT
 - Field instrumentation
 - FH tests on different material
 - Field Assessment
 - FS evaluation of FPL from 0-4 mm data

Other significant contribution

- **Loranger, B.**, Kuznetsova, E., Hoff, I., Aksnes, L. and Skoglund, K.A. (2017). Evaluation of Norwegian gradation-based regulation for frost susceptibility of crushed rock aggregates in roads and railways. *10th International Conference of the bearing capacity of roads, railways and airfield. Athens, Greece. (June 28th to 30th 2017)*.
- Rieksts, K., **Loranger, B.**, Hoff, I. and Scibilia, E. (2019). In situ thermal performance of lightweight aggregates expanded clay and foam glass in road structures. *Québec 2019: 18th International Conference on Cold Regions Engineering and the 8th Canadian Permafrost Conference. Quebec City, Canada. (August 18th to 22nd 2019)*.
- C. Kjelstrup, S., Ghoreishian Amiri, S.A., **Loranger, B.** and Grimstad, G. (2020). Transport coefficients and pressure conditions for growth of ice lens in frozen soil. *Acta Geotechnica journal (doi.org/10.1007/s11440-021-01158-0)*.



Thank you!
Takk!